COUN

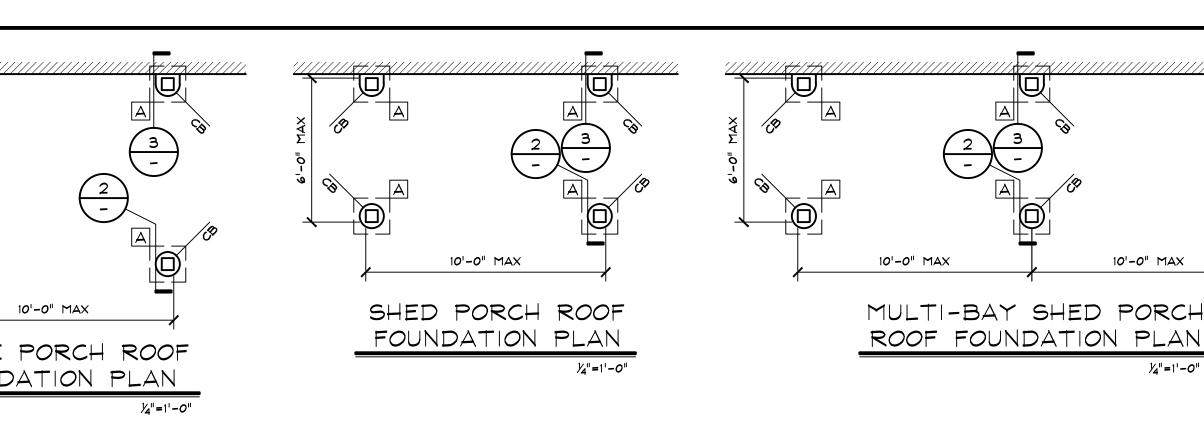
comdev@mono.ca.go www.monocounty.ca.go 9-17-12 3/4"=1'-0"

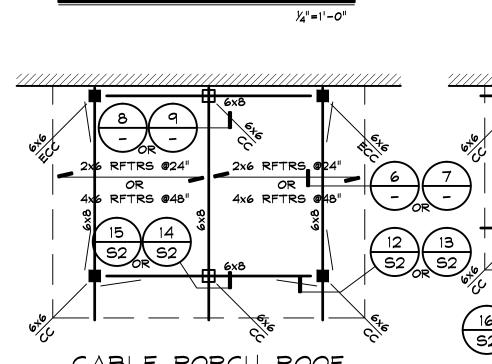
TMS

12-008C

REVISIONS







B.N. ~

CONTINUOUS -

OR BLK IF

BEYOND BM

EXTEND

3-16d/BLK,

U.N.O.

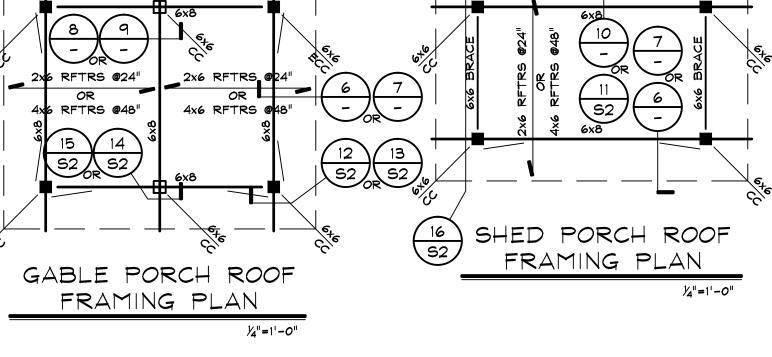
RIM MEMBER,

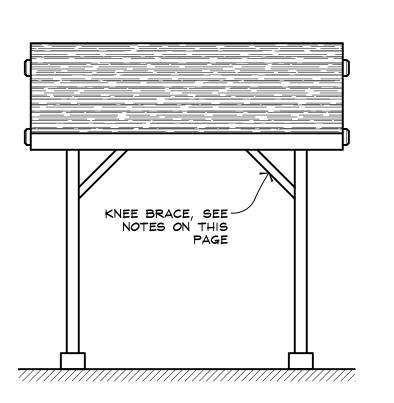
RFTRS MUST

- PLY 0/ 2x

RF-EXT_CRFTRL-RFTRR-BM_X_S-PERP.

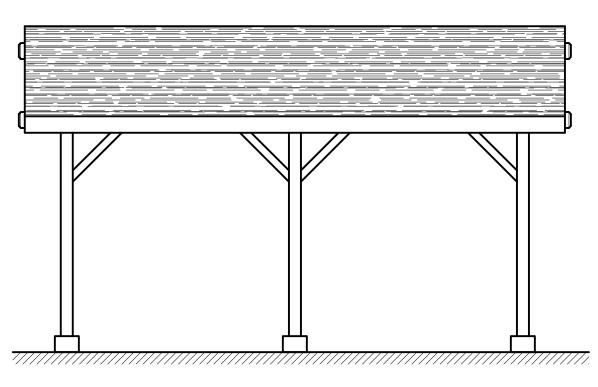
RFTRS @24"





SHED PORCH ROOF

FRONT ELEVATION



MULTI-BAY SHED PORCH

ROOF FRAMING PLAN

10'-0" MAX

S2

 $\frac{1}{2} || = 1^{1} - 0^{11}$

火"=1'-0"

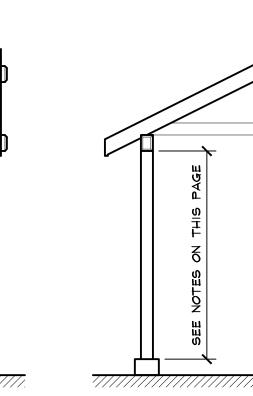
NOTES TO SUBMITTER

-KNEE BRACE, SEE 52

AND NOTES ON THIS

PAGE FOR WHEN THEY MAY BE

OMITTED



THESE PRESCRIPTIVE DESIGNS ARE INTENDED TO APPLY TO THE MOST

COMMON SITUATIONS ENCOUNTERED IN MONO COUNTY. HOWEVER, UNIQUE

SITE CONDITIONS OR SUBSTANTIAL DEVIATIONS FROM THESE DESIGNS AS

THESE PLANS ARE PRIMARILY FOR THE STRUCTURAL REQUIREMENTS OF

ARCHITECTURAL PLAN, SHOWING THE ACTUAL LAYOUT OF THE PORCH AND

ROOF. THE PLAN SHALL ALSO SHOW A STRUCTURAL LAYOUT BASED UPON

REQUIREMENTS, INCLUDING FLOOD PLAIN ZONES, CAL-FIRE WILDLAND URBAN

INTERFACE REQUIREMENTS, LAHONTAN EROSION CONTROL REQUIREMENTS

THIS DESIGN IS INTENDED NOT TO CONFLICT WITH AN EXISTING ROOF. IT

IS INTENDED TO GO ON A GABLE END WALL (A RAKE WALL), OR ON A LOWER STORY OF A 2 STORY STRUCTURE WHERE POSSIBLE. IF THIS

EAVE LINE, ONE OF THE FOLLOWING CONDITIONS MUST BE MET. THE

INTEGRITY AND ABILITY TO HANDLE ADDITIONAL LOADS IMPOSED BY

ALTERNATIVELY, IF THERE ARE EXISTING PLANS FOR THE STRUCTURE

REDUCTIONS FOR PITCH OR DURATION, THEN THE NEW PORCH ROOF

EXISTING ROOF STRUCTURE MUST BE EVALUATED BY A LICENSED

PROVIDE A DESIGN TO REINFORCE THE EXISTING STRUCTURE.

20/S2 AT GABLE ROOFS AND 21/S2 AT SHED ROOFS.

DESIGN NEEDS TO BE INSTALLED SUCH THAT IT IS EITHER AN EXTENSION OF AN EXISTING EAVE LINE, OR A GABLE COMING FROM AND EXISTING

PROFESSIONAL (ARCHITECT OR ENGINEER) TO EVALUATE ITS STRUCTURAL

VALLEYS, CHANGES IN PITCH, ETC. THE LICENSED PROFESSIONAL THEM MUST EITHER CERTIFY THAT THE EXISTING STRUCTURE IS ADEQUATE OR

AVAILABLE, AND THEY CAN DEMONSTRATE (FROM EITHER ORIGINAL PLANS

OR CALCULATIONS) THAT THE EXISTING ROOF STRUCTURE MEETS CURRENT

MONO COUNTY SNOW AND DEAD LOAD REQUIREMENTS FOR ROOFS, WITH NO

SHOULD NOT CAUSE DISTRESS BEYOND DESIGN LIMITS FOR THE EXISTING

IF A ROOF CONFLICT IS ALLOWED AND IS GOING TO OCCUR, SEE DETAIL

PORCH ROOFS. THE SUBMITTER IS RESPONSIBLE FOR PREPARING AN

LASTLY THE SUBMITTER IS RESPONSIBLE FOR ALL SITE SPECIFIC

DETERMINED BY THE BUILDING OFFICIAL MAY WARRANT ADDITIONAL

ARCHITECTURAL OR STRUCTURAL DESIGN REQUIREMENTS.

NEW ROOFS THAT CONFLICT WITH EXISTING ROOFS

THE REQUIREMENTS OF THESE PLANS.

AND ANY SIMILAR REQUIRÉMENTS.

ROOF STRUCTURE.

SHED PORCH ROOF

SIDE ELEVATION

1/4"=1'-0"

MONO

SUCH USE.

COUNTY PROVIDES

THESE PLANS TO THE PUBLIC

AS A COURTESY AND WITHOUT

ANY WARRANTIES, EXPRESS OR IMPLIED, REGARDING THEIR FITNESS FOR ANY PARTICULAR

APPLICATION. AMONG OTHER

THINGS, MONO COUNTY DOES

NOT REPRESENT OR WARRANT

THAT THE DESIGNS WITHIN SAID PLANS ARE FREE FROM

FLAWS OR DEFECTS. ANYONE

UTILIZING THESE PLANS DOES

SO AT THEIR OWN RISK AND

WAIVES ANY CLAIMS AGAINST

MONO COUNTY ARISING FROM

MULTI-BAY SHED PORCH FRONT ELEVATION PLAN

1/4"=1'-0" DEFINITION OF A PORCH ROOF

A PORCH ROOF IS A SMALL STRUCTURE THAT COVERS AN ENTRY OR OTHER SMALL AREA NEEDING PROTECTION. ONE SIDE IS AGAINST AN EXISTING BUILDING AND THE OTHER THREE SIDES ARE OPEN.

S2/

FLOOR OF THE PORCH

A 4" SLAB ON GRADE MAY BE USED AS THE FLOOR OF THE PORCH. IT SHOULD BE REINFORCED WITH #3@24" S.E.E.W. IF THE DESIRE IF FOR A RAISED PORCH/ENTRY, STILL OF CONCRETE, WITH STAIRS, SEE DETAIL 4/SI AND 5/SI FOR STAIRS AND PROTECTION OF CONCRETE IN CONTACT WITH EXISTING FRAMING. IF A DECK IS DESIRED, THAT IS BEYOND THE SCOPE OF THESE PLANS. HOWEVER, MONO COUNTY BUILDING DIVISION HAS PRESCRIPTIVE DESIGNS FOR DECKS THAT CAN BE COMBINED WITH THIS PORCH ROOF DESIGN. HEIGHT OF POSTS

MAXIMUM HEIGHT OF SUPPORT POSTS (OR LOW POSTS IN THE CASE OF THE SHED PORCH ROOFS) IS 10' AS DRAWN. HOWEVER, IF THERE IS A DECK, AND THE DECK IS TIED IN (NAILING OR SCREWING A DECK JOIST TO A POST IS SUFFICIENT) TO THE POST, THEN THE POST MAY HAVE A MAXIMUM HEIGHT OF 14', BUT THE MAXIMUM UNRESTRAINED HEIGHT OF THE POST (FROM THE DECK TO THE BEAM) IS STILL 10'

KNEE BRACING

THE KNEE BRACING IS INCLUDED TO PROVIDE LATERAL SUPPORT TO THESE MINOR STRUCTURES AND ELIMINATE OR MITIGATE ANY LATERAL LOADING THIS STRUCTURE MIGHT IMPOSE ON AN EXISTING STRUCTURE. IF THE OWNER DOES ONE OF TWO THINGS, THE KNEE BRACES MAY BE ELIMINATED. IF THE OWNER HIRES A LICENSED PROFESSIONAL (AND ENGINEER OR ARCHITECT) TO VERIFY THAT THE EXISTING STRUCTURE HAS ADEQUATE RESISTANCE TO ANY LOADS IMPOSED BY THE NEW ENTRY PORCH. ALTERNATIVELY, IF THE OWNER HAS ACCESS TO EXISTING PLANS THAT DEMONSTRATE THAT THE EXISTING STRUCTURE WAS DESIGNED TO CURRENT CODES FOR SEISMIC RESISTANCE (AS FAR AS LOADING), THEN KNEE BRACES CAN BE ELIMINATED. GENERALLY ANY STRUCTURE PROPERLY DESIGNED FOR THE 1994 U.B.C. OR LATER VERSIONS OF THE U.B.C., C.B.C., OR I.B.C. WILL HAVE ADEQUATE LATERAL RESISTANCE. NOTE TO THAT THIS MUST BE A SYSTEM CONSISTING OF STRUCTURAL SHEAR PANELS IN THE LINE TO WHICH THE PORCH ROOF IS BEING ADDED. OTHER SYSTEMS (MOMENT FRAMES, HARDY FRAMES, SIMPSON STRONG WALLS, OR SIMILAR) MUST BE LOOKED AT BY AND LICENSED PROFESSIONAL OR KNEE BRACES ARE REQUIRED.

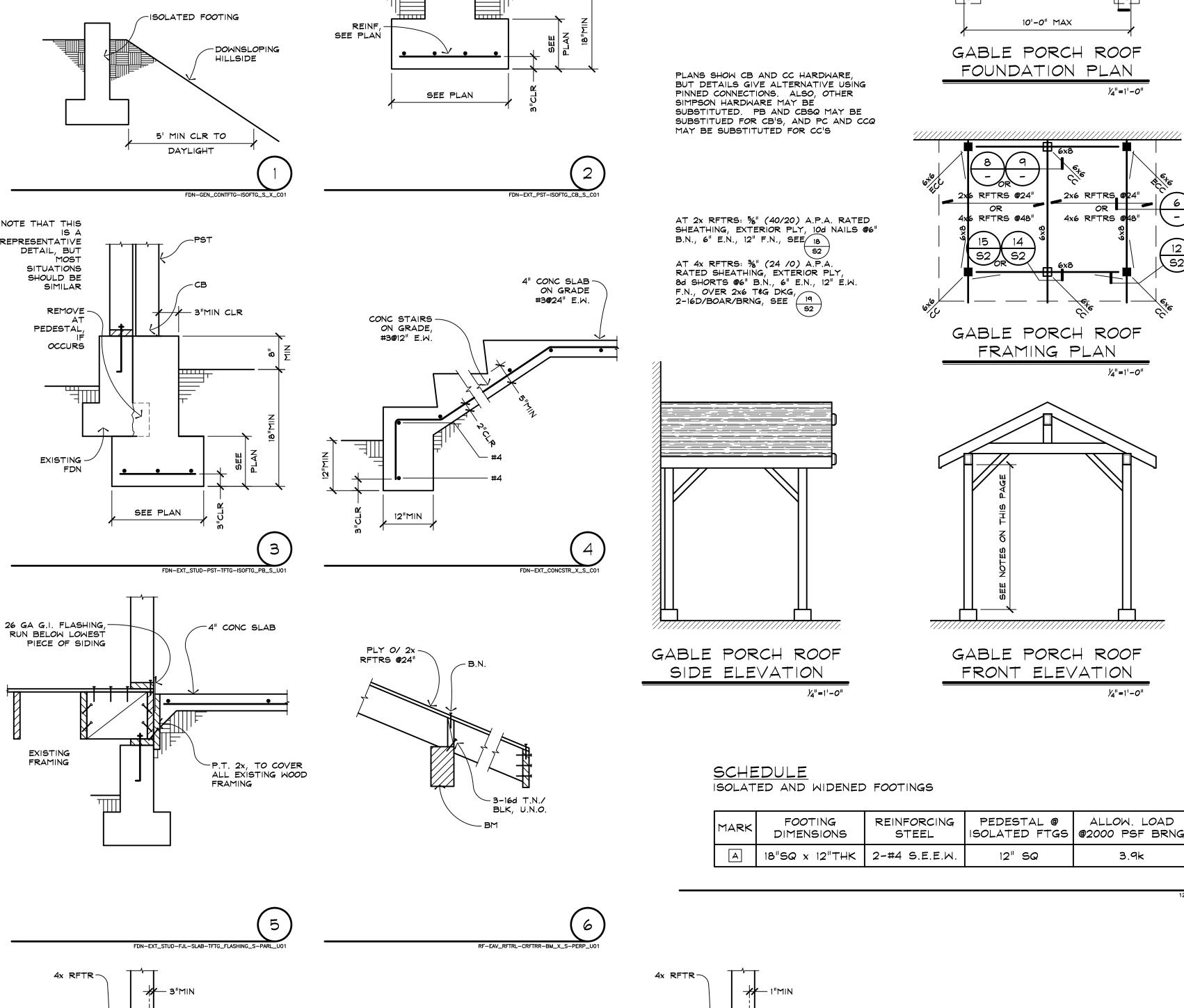
> STRUCTURAL NOTES PROJECT SHALL COMPLY WITH THE 2010 CALIFORNIA CODES, WHICH ARE BASED UPON THE 2009 INTERNATIONAL BUILDING CODE, THE 2009 INTERNATIONAL RESIDENTIAL CODE, THE 2009 UNIFORM PLUMBING CODE, THE 2009 UNIFORM MECHÁNICAL CODE, THE 2008 NATIONAL ELECTRICAL CODE, AND THE 2008 TITLE 24 ENERGY STANDARDS. SOIL BEARING ALLOWABLE ASSUMED TO BE 2000 PSF. ALL EXTERIOR FOOTINGS SHALL HAVE 18" MIN EMBEDMENT ALL FOOTINGS SHALL ALSO BE EMBEDDED DEEP ENOUGH THAT A 5' MIN HORIZONTAL DISTANCE TO DAYLIGHT IS ATTAINED. SEE

WHERE SOLID-FILLED POSTS ARE CALLED OUT, THE SOLID FILL REPRESENTS A POST BELOW. WHERE OPEN NON-FILLED POSTS ARE CALLED OUT, THE OPEN POST REPRESENTS A POST ABOVE.

PB, CC, ETC ARE SIMPSON STRONG-TIE HARDWARE. REFER TO SIMPSON CATALOG C-2011 FOR INSTALLATION INFORMATION. USE EXACT TYPE, SIZE, AND NUMBER OF FASTENERS SPECIFIED IN CATALOG.

DETAILS ON ACCOMPANYING DETAIL SHEETS ARE DRAWN TO THE SCALE NOTED IN THE TITLE BLOCK OF THE SHEET, U.N.O. HOWEVER, THE SIZE OF EACH SCALED ELEMENT SHOWN ON THE DETAILS DOES NOT NECESSARILY

REPRESENT THE SIZE OF THE MEMBERS CALLED OUT ON THE PLAN, OR EXISTING IN THE STRUCTURE.



2x BLKG

-3-16d T.Ń.∕

BLK, U.N.O.

RF-RDG_RFTRL-RFTRR-BM_LAP_S-PERP_U

-ST6224,

RFTR-TO-RFTR

-ATTACH 4x RFTR TO BM W/

3"Φ POWER PRO CERAMIC

COATED LAG SCREWS, SELECT

LENGTH TO ALLOW FOR 6" MIN

EMBEDMENT IN BM PER RFTR,

OFFSET APPROXIMATELY AS

SHOWN IN PLAN VIEW

RF-RDG_T&G-4xRFTRL-4xRFTRR-BM_ST6224-LAG_P-S-PERP_U

PLY # DKG # ST

STRAP OMITTED

FROM PLAN VIEW

PLY 0/ 2x DKG~

O/ 4x RFTRS

@48"(MAX)

FOR CLARITY

PEDESTAL

-CONTINUOUS FOOTING

5' MIN CLR TO

DAYLIGHT

PLY & DKG OMITTED

FROM PLAN VIEW

PLY 0/ 2x DKG 0/~

4x RFTRS @48"(MAX)

FOR CLARITY

PLY 0/ 2x -

RFTRS @24"

-ATTACH 4x RFTR TO BM

W/ 2-3/4 POWER PRO

CERAMIC COATED LAG

TO ALLOW FOR 6" MIN

OFFSET APPROXIMÁTELY

AS SHOWN IN PLAN VIEW

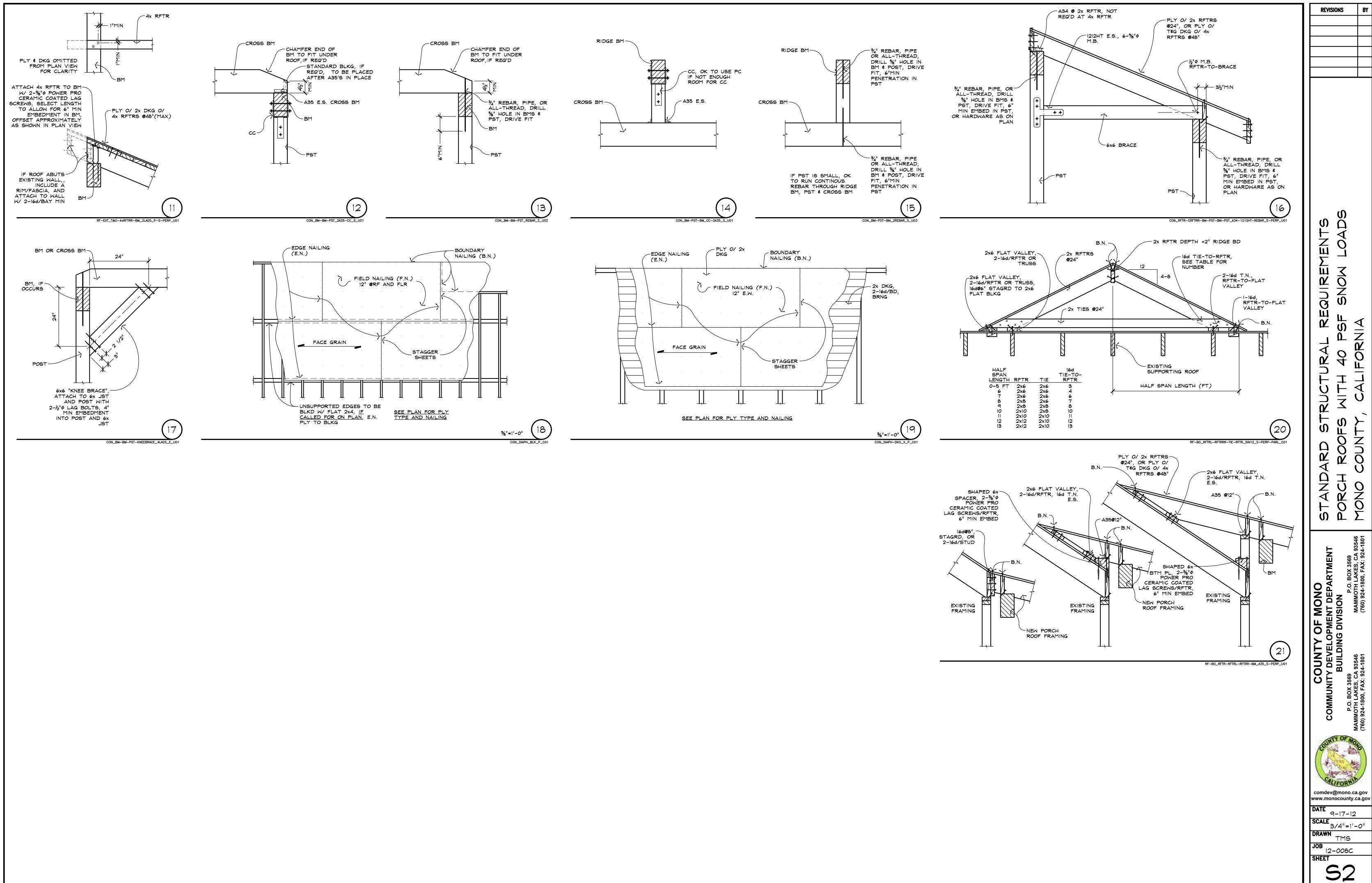
EMBEDMENT IN BM,

RF-EAV_T&G-4xRFTRL-4xCRFTRR-BM_2LAGS_P-S-PERP_U

SCREWS, SELECT LENGTH

DOWNSLOPING

HILLSIDE



N O STANDARD PORCH ROC MONO COUN

CODES AND REFERENCES

- A. ALL MATERIALS AND WORKMANSHIP SHALL CONFORM TO THE REQUIREMENTS OF THE 2010 CAL-IFORNIA BUILDING CODE (C.B.C.) BASED UPON THE 2009 INTERNATIONAL BUILDING CODE (I.B.C.)
- B. A THOROUGH PLANCHECK SHALL BE MADE BY A QUALIFIED REPRESENTATIVE OF THE BUILDING DEPARTMENT PRIOR TO THE ISSUANCE OF A BUILDING PERMIT. CORRECTIONS, IS ANY, SHALL BE MADE ONLY BY THE ARCHITECT OR HIS REPRESENTATIVE. ONCE THE BUILDING PERMIT HAS BEEN ISSUED NO CHANGES OR DEVIATIONS SHALL BE MADE WITHOUT THE WRITTEN APPROVAL OF THE ARCHITECT, LEST AN UNSAFE OF UNLAWFUL CONDITION BE CREATED. CONTRACTOR SHALL COMPLY WITH ANY CODE OR LEGAL VIOLATION WHICH MIGHT BE POINTED OUT BY THE BUILDING INSPECTOR.
- C. WHERE REFERENCE IS MADE TO VARIOUS TEST STANDARDS FOR MATERIALS, SUCH STANDARDS SHALL BE THE LATEST EDITION, AND/OR ADDENDUM. THESE STANDARDS WILL BE REFERRED TO IN ABBREVIATED FROM AS LISTED BELOW:
 - AMERICAN CONCRETE INSTITUTE
 - AMERICAN FOREST AND PAPER ASSOCIATION AMERICAN INSTITUTE OF STEEL CONSTRUCTION
 - AMERICAN INSTITUTE OF TIMBER CONSTRUCTION
 - AMERICAN NATIONAL STANDARDS INSTITUTE AMERICAN PLYWOOD ASSOCIATION
 - AMERICAN SOCIETY OF TESTING MATERIALS
 - AMERICAN WELDING SOCIETY
 - INTERNATIONAL CODE COUNCIL
 - WEST COAST LUMBER INSPECTION BUREAU WWPA WESTERN WOOD PRODUCTS ASSOCIATION
- D. CONTRACTOR SHALL CHECK AND VERIFY ALL DIMENSIONS AND CONDITIONS ON THE JOB SITE AND REPORT ANY ERRORS, OMISSIONS, OR POSSIBLE DISCREPANCIES TO THE ARCHITECT PRIOR TO PROCEEDING WITH THE WORK. SPECIAL CARE SHALL BE GIVEN SITE AND BUILDING LAYOUT THEREUPON.
- E. TYPICAL DETAILS AND NOTES SHALL APPLY UNLESS SHOWN OTHERWISE ON THE PLANS.

2. SPECIAL INSPECTION

WHERE "SPECIAL INSPECTION" IS REQUIRED ON THE PLANS, A REGISTERED DEPUTY INSPECTOR APPROVED BY, AND RESPONSIBLE TO, THE ARCHITECT AND THE BUILDING DEPARTMENT, SHALL BE EMPLOYED BY THE OWNER. SPECIAL INSPECTION IS REQUIRED FOR:

- A. PLACING OF ALL CONCRETE WITH AND f'c IN EXCESS OF 2500 PSI.
- B. ALL FIELD WELDING, OR WELDING PERFORMED IN AN UNLICENSED FABRICATING SHOP.
- C. ALL CERTIFIED COMPACTED FILL.
- D. SUCH OTHER ITEMS AS MAY BE REQUIRED BY CHAPTER 17 OF THE C.B.C. OR BY THE ARCHITECT.

3. TEMPORARY BRACING

THE CONTRACTOR SHALL PROVIDE SAFE AND ADEQUATE BRACES AND CONNECTIONS TO SUPPORT THE COMPONENT PARTS OF THE STRUCTURE UNTIL THE STRUCTURE ITSELF (INCLUDING THE FLOOR AND ROOF DIAPHRAGMS) IS COMPLETE ENOUGH TO ADEQUATELY SUPPORT ITSELF. CONCRETE OR MASONRY WALLS ARE NOTED IN PARTICULAR.

4. SHOP (OR FABRICATION) DRAWINGS, DESIGNS

A. AS A CONVENIENCE TO THE CONTRACTOR, ARCHITECT SHALL REVIEW REQUIRED SHOP DRAWINGS AS TO THEIR GENERAL CONFORMANCE TO THE DESIGN CONCEPT. CONTRACTOR SHALL BE RESPONSIBLE, NONETHELESS, FOR COMPLIANCE AND DIMENSIONS AND SHALL SUBMIT SHOP DRAWINGS, IF APPLICABLE, FOR THE FOLLOWING: (REBAR PLACING DRAWINGS NOT REQUIRED)

1. GLULAM BEAMS AND PANELIZED ROOF FRAMING.

- 2. STRUCTURAL STEEL AND TAPERED STEEL GIRDERS.
- 3. ERECTION BRACING AND SEQUENCE.
- 4. PRECAST CONCRETE ELEMENTS, INCLUDING PICKUP POINTS, STRONG BACKS AND BRACING, ALL CERTIFIED BY A REGISTERED CIVIL OR STRUCTURAL ENGINEER.
- 5. CONCRETE POURING SEQUENCE, SHORING DETAILS AND SPECIAL CONSTRUCTION TECHNIQUES (ARCHITECT OR CIVIL OR STRUCTURAL ENGINEER'S CERTIFICATION MAY
- 6. SUCH OTHER ITEMS AS MAY BE REQUIRED ON PLANS.
- CONTRACTOR SHALL SUBMIT SHOP DRAWINGS AND COMPLIANCE CERTIFICATES TO THE BUILDING DEPARTMENT WHEN REVIEWED.
- B. WHERE DESIGN AND DETAILS OF PLATE GIRDERS, TRUSSES, etc. ARE TO BE PROVIDED BY FABRICATOR, CONTRACTOR SHALL SUBMIT CALCULATIONS AND DRAWINGS PREPARED AND CERTIFIED BY AN ARCHITECT, OR A CIVIL OR STRUCTURAL ENGINEER TO THE ARCHITECT AND TO THE BUILDING DEPARTMENT FOR REVIEW PRIOR TO FABRICATION.

5. OPTIONS AND SUBSTITUTIONS

- A. OPTIONS, IF PROVIDED HEREIN, ARE BOTH FOR CONTRACTOR'S CONVENIENCE AND THE OWNER'S ADVANTAGE. "SUBSTITUTIONS." IF SUGGESTED BY THE CONTRACTOR. MUST BE APPROVED BY BOTH THE ARCHITECT AND THE OWNER AND SHALL NOT DIMINISH THE DEGREE OF QUALITY INTENDED ON THE PLANS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL CHANGES NECESSARY, SHALL COORDINATE ALL DETAILS, AND SHALL OBTAIN ALL REQUIRED APPROVALS.
- B. COSTS OF ADDITIONAL ARCHITECT'S DESIGN OR DETAIL WORK NECESSITATED BY SELECTION OF AN OPTION, AS A RESULT OF A SUBSTITUTION, OR DUE TO ERRORS OR OMISSIONS IN CONSTRUCTION, SHALL BE BORNE BY THE CONTRACTOR.

6. PROTECTION BY CONTRACTOR

- A. CONTRACTOR SHALL LOCATE ALL EXISTING UTILITIES WHETHER OR NOT SHOWN ON THE DRAWINGS AND PROTECT THEM FROM DAMAGE.
- B. HE SHALL COMPLY WITH ALL LAWS AND REGULATIONS REGARDING PROTECTION OF THE PUBLIC AND THE WORKMEN DURING CONSTRUCTION.
- C. HE SHALL BEAR ALL EXPENSE OF REPAIR OR REPLACEMENT RELATIVE TO THE PROSECUTION OF THIS WORK.

FOUNDATION (C.B.C. CHAPTER 18):

- 1. SEE FOUNDATION PLAN FOR COMPLETE DATA: DESIGN SOIL PRESSURE, FOUNDATION DEPTH etc, AND REFERENCE TO "SOIL REPORT." WHERE "SOIL REPORT" IS CITED, IT SHALL BE A PART OF THESE PLANS AND ALL OF ITS REQUIREMENTS AND RECOMMENDATIONS SHALL BE PERFORMED BY THE CONTRACTOR WHO SHALL OBTAIN A COPY OF SAID REPORT. IN ABSENCE OF SOIL REPORT AND INSPECTION BY SOIL ENGINEER, CONTRACTOR SHALL NOTIFY ARCHITECT IF HE ENCOUNTERS ANY UNUSUAL SOIL CONDITIONS (SOFT OR UNSTABLE SOIL, WET SOIL, etc).
- 2. SLABS ON GRADE: PROVIDE CONSTRUCTION OR CRACK-CONTROL JOINTS SPACED NO FARTHER THAN 15' APART. SLAB AREAS PLACED SHALL NOT EXCEED 225 SQUARE FEET.

CONCRETE AND EMBEDDED ITEMS (C.B.C. CHAPTER 19):

- 1. ALL CONCRETE SHALL BE MIXED, FORMED AND PLACED ACCORDING TO THE AMERICAN CONCRETE INSTITUTE (ACI) BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE, ACI 318-08.
- DAYS. USE 6 SACKS OF CEMENT (MINIMUM) PER YARD OF CONCRETE FOR WEATHER DURABILITY. EXCEPTIONS SHALL BE NOTED HEREIN OR ON PLANS.

2. CONCRETE SHALL HAVE A MINIMUM ULTIMATE COMPRESSIVE STRENGTH OF 2500 PSI AT 28

- 3. CEMENT FOR CONCRETE SHALL BE A STANDARD BRAND "PORTLAND CEMENT," MEETING THE REQUIREMENTS OF ASTM C150, TYPE I OR II, LOW ALKALI.
- 4. AGGREGATES FOR CONCRETE SHALL MEET THE REQUIREMENTS OF ASTM C33.
- 5. CONCRETE SHALL BE MACHINE-MIXED USING A MAXIMUM OF 7½ GALLONS OF WATER PER SACK OF CEMENT. READYMIX CONCRETE SHALL BE MIXED AND DELIVERED IN ACCORDANCE
- 6. CONTRACTOR MAY USE A WATER REDUCING ADMIXTURE CONFORMING TO ASTM C494, PROVIDED ARCHITECT IS NOTIFIED IN WRITING IN ADVANCE OF ITS USE.
- 7. ALL REINFORCING STEEL, ANCHOR BOLTS, DOWELS EMBEDDED PIPES AND CONDUIT SHALL BE SECURELY FASTENED IN THE FORMS BEFORE CONCRETE IS POURED. ADEQUATE CLEANOUTS SHALL BE PROVIDED IN THE BOTTOM OF THE CONCRETE FORMS FOR PROPER CLEANING AND INSPECTION.
- 8. SLABS POURED ON GRADE SHALL BE LEVEL (OR PLANAR) TO WITHIN 1/2" IN 8'-0" IN ANY DIRECTION EXCEPT AS NOTED OTHERWISE ON PLANS. WALLS SHALL BE SIMILARLY ACCURATE, AS SHALL OTHER SLABS SUPPORTED ON FORMS.
- 9. AT ALL OPENINGS IN CONCRETE PROVIDE TWO #4 BARS, UNLESS NOTED OTHERWISE, AT JAMBS, HEAD AND SILL, EXTENDING 2'-0" BEYOND EDGES OF OPENING.
- 10. MINIMUM EMBEDMENT OF ANCHOR BOLTS (A.B.) SHALL BE 7" IN HORIZONTAL CONCRETE SURFACES (FOOTINGS, etc) AND 4" INTO VERTICAL CONCRETE SURFACES (WALLS, etc). ALL BOLTS SHALL HAVE A 4 DIAMETER, 90° BEND AT EMBEDDED END. ANCHOR BOLTS SHALL BE SPACED 12 DIAMETERS, MINIMUM.
- 11. EXPANSION BOLTS, ITW RAMSET/"RED HEAD," etc, MAY BE USED IN LIEU OF CAST-IN-PLACE BOLTS WHERE SPECIAL CONDITIONS WARRANT THEIR USE, PROVIDED WRITTEN APPROVAL OF THE ARCHITECT IS OBTAINED.
- 12. FOOTING DOWELS SHALL MATCH VERTICAL WALL STEEL. LAP 36 DIAMETERS, MINIMUM.
- 13. CEMENT GROUT AND DRY-PACK GROUT SHALL CONSIST OF 1 PART CEMENT TO 2½ PARTS FINE AGGREGATE BY VOLUME. ADD SUFFICIENT WATER TO MAKE THE MIXTURE FLOW UNDER ITS OWN WEIGHT. FOR USE AS DRY-PACK CONCRETE (HAND-PLACED BELOW METAL OR WOOD PLATES) ADD WATER TO MAKE A STIFF MIXTURE WHICH CAN BE MOLDED INTO A SPHERE. GROUT SHALL ATTAIN A MINIMUM ULTIMATE COMPRESSIVE STRENGTH OF 2500 PSI AT 28 DAYS.
- 14. RETAINING WALLS SHALL NOT BE BACKFILLED UNTIL 7 DAYS AFTER PLACEMENT OF CONCRETE.

REINFORCING STEEL (C.B.C. CHAPTER 19):

- 1. ALL REINFORCING STEEL SHALL MEET THE REQUIREMENTS OF, AND BE PLACED IN ACCORDANCE WITH, THE AMERICAN CONCRETE INSTITUTE (ACI) BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE, ACI 318-08.
- 2. REINFORCING STEEL SHALL BE INTERMEDIATE GRADE DEFORMED U.N.O. (EXCEPT #2 TIES OR STIRRUPS) BARS CONFORMING TO ASTM A615, GRADE 40 TYPICALLY. LAP BARS A MINIMUM OF 30 DIAMETERS. WHERE GRADE 60 (HARD GRADE) IS REQUIRED ON PLANS, LAP 36 DIAMETERS. STAGGER LAPS WHERE PERMISSIBLE.
- 3. ALL WELDED REBAR TO BE GRADE A706.
- 4. WIRE MESH SHALL CONFORM TO ASTM A185. LAP 8" MINIMUM.
- 5. LOW HYDROGEN, E70 SERIES, WELDING RODS SHALL BE USED FOR ALL WELDING OF REINFORCING BARS COMPLYING WITH AWS D1.4.
- 6. PROVIDE DOWELS IN FOOTINGS AND/OR GRADE BEAMS THE SAME SIZE AND NUMBER AS VERTICAL WALL REINFORCING. PROJECT DOWELS EQUAL TO STANDARD LAP SPLICE AND WIRE TO VERTICAL STEEL.
- 7. #5 OR LARGER REBAR SHALL NOT BE RE-BENT WITHOUT APPROVAL.
- 8. MINIMUM CONCRETE COVER SHALL BE:
 - CONCRETE POURED AGAINST EARTH, BOTTOM AND SIDES.
 - FORMED CONCRETE WHICH WILL REMAIN IN CONTACT WITH EARTH, INCLUDING STEEL IN TOP SURFACES OF FOOTINGS AND WALL SURFACES IN CONTACT WITH FARTH
 - BEAMS, MEASURED TO MAIN STEEL: COLUMNS, MEASURED TO TIES OR SPIRALS: EXPOSED FACES OF WALLS ABOVE GRADE OR THEIR SURFACES NOT IN CONTACT WITH EARTH.
 - TOP SURFACES OF SLABS DIRECTLY EXPOSED TO THE ELEMENTS.
 - INTERIOR SLABS: INSIDE FACES OF WALLS.

WOOD CONSTRUCTION (C.B.C. CHAPTER 23):

1. STRUCTURAL LUMBER SHALL BE GRADE-MARKED DOUGLAS FIR-LARCH (Df-L) PER STANDARD GRADING RULES NO. 17, WCLIB, AND STANDARD GRADING RULES, WWPA.

JOISTS, BEAMS, PURLINS AND POSTS 4" AND WIDER	<u>GRADE</u> NO. 1
JOISTS AND SUB-PURLINS 2" WIDE, 2x6 OR DEEPER STUDS, TOP PLATES, SILL PLATES AT BEARING WALLS, AND LEDGERS OF ALL WIDTHS	NO. 2
2x4 AND 3x4 STUDS	STUD
BLOCKING, NON-BEARING SILL PLATES AND MISC.	NO. 3

- 2. COMMON NAILS SHALL BE USED. BOX NAILS, IF INCREASED IN NUMBER BY 33%, MAY ALSO BE USED.
- 3. SILLS OR PLATES BEARING ON CONCRETE OR MASONRY WHICH IS WITHIN 48" OF EARTH SHALL BE PRESSURE TREATED (P.T.). SILLS SHALL BE BOLTED TO THE FOUNDATION WITH %" DIAMETER x 10" BOLTS AT 4'-0" O.C., 12" MIN, FROM ENDS, OR 2 BOLTS MIN PER PIECE,
- 4. FIREBLOCKING, 2" THICK, SHALL BE PLACED IN STUD WALLS AT CEILING AND FLOOR LEVELS, AT EACH 10' HEIGHT OF STUDS, AND BETWEEN STAIR STRINGERS AT SUPPORTS.
- 5. JOISTS AND RAFTERS SHALL BE BLOCKED AT SUPPORTS AND BRIDGED OR BLOCKED AT INTERVALS OF 8' WHERE JOISTS ARE 2x12'S OR DEEPER.
- 6. JOISTS UNDER BEARING PARTITIONS (ONE STORY ABOVE) SHALL BE DOUBLED; TRIPLED FOR TWO STORIES ABOVE.
- 7. PLYWOOD SHALL BE PER APA PS 1-07. PROVIDE A $\frac{1}{2}$ " SPACE BETWEEN ALL JOINTS.

0.	DEPTH AND SCREWED (NOT DRIVEN) INTO PLACE.	SHANK DIAMETER TO FULL	ADDITEVIATION	
9.	CUT WASHERS SHALL BE PLACED UNDER HEADS AND NUTS OF ALL HEADS OF LAGBOLTS. CUT WASHER SHALL BE USED FOR BOLTS LEDGERS TO CONCRETE OR MASONRY WALLS.	BOLTS AND UNDER CONNECTING WOOD	A.B. ALT ARCHL B, BOT	ANCHOR BOLT ALTERNATE(ING) ARCHITECTURAL BOTTOM
10.	WHERE REQUIRED IN ALL CONDITIONS EXCEPT SILL PLATE ANCHOR (M.I.WA.) OR PLATE (PL.WA.) WASHERS SHALL BE SIZED AS FOLLOW		B.C. B.N.	BOTTOM CHORD BOUNDARY NAILING
	BOLT Ø M.I.WA. MASTIERS SHALL BE SIZED AS FOLLOW PL.WA.	3.	BLK BLKD BLKG	BLOCK BLOCKED BLOCKING
	½" ¼"×2½"ø ¾6"×2" SQ %" ¼6"×2¾" ¼"×2½"		BM BRNG	BEAM BEARING
	78 716 ^274 74 ^272 34" 76"×3" 516"×234" 78" 76"×3½" 516"×3"		C.B.C. CLR	CALIFORNIA BUILDING CODE CLEAR
	1" ½"×3¾" ¾"×3½"		COL CONC	COLUMN CONCRETE
11.	SEE NOTES BELOW SHEAR PANEL SCHEDULE FOR REQUIREMENTS FOR PLATE ANCHOR BOLTS.	OR WASHERS AT SILL	CONT CONST	CONTINUOUS CONSTRUCTION
12.	ALL STRUCTURAL PLYWOOD NAILING (ROOF, FLOOR AND WALLS) SHABUILDING INSPECTOR PRIOR TO COVERING.	ALL BE INSPECTED BY THE	CSK DBL	COUNTERSUNK DOUBLE
13.	3. STUDS IN BEARING WALLS SHALL NOT BE NOTCHED UNLESS SPECIFICALLY DETAILED BY ARCHITECT.		DET DIAM, Ø DIM	DETAIL DIAMETER DIMENSION
14.	FRAMING HARDWARE SHALL BE SIMPSON STRONG—TIE $^{\$}$. REFER TO FOR INSTALLATION INFORMATION. USE EXACT TYPE, SIZE AND NUM SPECIFIED IN CATALOG.		DKG do Df-L DWG	DECKING DITTO DOUGLAS FIR—LARCH DRAWING
15.	REFER TO THE FOLLOWING ICC REPORTS FOR SIMPSON CONNECTOR ER4935— SSTB, HCA, MSTC	S	EA E.F.	EACH EACH FACE
	ER4945— EPOXY TIE ANCHORING SYSTEMS WITH ET, ETF, AND ETR ER5090— ANCHOR TIEDOWN SYSTEMS	ANCHORING ADHESIVES	E.N. E.S.	EDGE NAILING EACH SIDE
	ER5952— CBSQ—SDS2 AND CBQ—SDS2 COLUMN BASE CONNECTORS	S AND ECCQ/CCQ-SDS2 COLUMN	E.W. EMBED	EACH WAY EMBEDMENT
	NER393— ETA/TSS, MAB, HIT, JB/LB, PF, LU, LUP, LTT/LTTI, HA/F LCB/CB, PA/PAI/PAT/PATM/PAR/PARP, MPAI, HPA, HPAT		ETC EQ	ET CETERA EQUAL
	NER432— ABE, CBA, EPB44T, H2.5, H10—2, H15, H15—2, HGT—2, LTHJ, LTP4, LTT131, MSC, RSP4, SP, SS, THG2A, TWB		EX, EXIST EXT FLG	EXISTING EXTERIOR FLANGE
	ESR-1056- TITEN HD ESR-1267- STRONG-WALL SHEAR PANELS		F.F. F.G.	FINISH FLOOR FINISH GRADE
	ESR-1396- WEDGE-ALL ANCHORS		F.J. F.N.	FLOOR JOIST FIELD NAILING
	ESR-1472- QUICK DRIVE WSNTL WOOD SCREWS ESR-1679- STEEL STRONG-WALL SHEAR PANELS		FLR FT	FLOOR FOOT
	ESR-1771- STRONG-BOLT WEDGE ANCHOR ESR-1772- SET EPOXY		G.I. GA	GALVANIZED IRON GAUGE
	ESR-1886- LBV, B, HB, AND BA SERIES JOIST HANGERS ESR-2105- TIE STRAPS		GLB GLULAM	GLUE—LAMINATED BEAM GLUE—LAMINATED
	ESR-2138- POWDER-ACTUATED FASTENERS ESR-2236- STRONG-DRIVE SDS SERIES WOOD SCREWS	(TUD. AND. ATUD.)	GRD HDR	GRADE HEADER
	ESR-2320 - COUPLING TAKE-UP DEVICE (CTUD) AND TAKE-UP DEVICESR-2508 - HOLD-DOWN CONNECTORS		HT H, HOR	HANGER HEIGHT HORIZONTAL
	ESR-2523- SET-XP EPOXY ADHESIVE ANCHORS FOR CRACKED AND ESR-2549- FACE-MOUNT HANGERS FOR WOOD FRAMING		I.D. INT	INSIDE DIAMETER INTERIOR
	ESR-2551- ADJUSTABLE HANGERS AND HIP CONNECTORS FOR WOO ESR-2552- FACE-MOUNT HANGERS SUPPORTING STRUCTURAL COM	POSITE LUMBER (SCL) AND	JST K.S.	JOIST KING STUD
	PREFABRICATED WOOD I—JOISTS (ENGINEERED WOOD PESR—2553— TOP—FLANGE HANGERS FOR SAWN LUMBER.		L LAG	ANGLE SHAPE LAGBOLT
	ESR-2553- TOP-FLANGE HANGERS FOR SAWN LUMBER. ESR-2554- MULTIPLE TRUSS HANGERS. ESR-2555- MASA/MASAP CAST-IN-PLACE FOUNDATION ANCHOR ST	RAPS.	LAM	LAMINATED LEDGER
	ESR-2604- COLUMN CAPS FOR WOOD CONSTRUCTION. ESR-2605- CONNECTORS FOR METAL PLATE CONNECTED WOOD TRUSS CONSTRUCTION. ESR-2606- STRUCTURAL ANGLES, CLIPS, AND PLATES FOR WOOD FRAMING. ESR-2607- CONNECTORS FOR PANELIZED ROOF CONSTRUCTION. ESR-2608- STUD SHOES, PLATE TIES, WALL BRACING, AND JOIST BRIDGING FOR WOOD CONSTRUCTION.			LONG MACHINE BOLT
	ESR-2606- STRUCTURAL ANGLES, CLIPS, AND PLATES FOR WOOD FRAMING. ESR-2607- CONNECTORS FOR PANELIZED ROOF CONSTRUCTION.			MAXIMUM MINIMUM
				MISCELLANEOUS NOT TO SCALE
	ESR-2611- STUD SHOES, PLATE TIES, WALL BRACING, AND JOIST BRIDGING FOR WOOD CONSTRUCTION.			OVER ON CENTER OUTSIDE DIAMETER
	ESR-2613- SSTB SERIES AND SB SERIES CAST-IN-PLACE ANCHOR ESR-2614- MISCELLANEOUS CONNECTORS.	R BOLTS.	O.D. OK OPT	OKAY OPTIONAL
	ESR-2613- SSTB SERIES AND SB SERIES CAST-IN-PLACE ANCHOR BOLTS. ESR-2614- MISCELLANEOUS CONNECTORS. ESR-2615- TOP-FLANGE HANGERS FOR ENGINEERED WOOD PRODUCTS (EWP). ESR-2616- CONNECTORS FOR WOOD MEMBERS SUPPORTED BY CONCRETE OR MASONRY			PARTITION PLASTER
	ESR-2713- TITEN HD SCREW ANCHOR AND TITEN HD ROD HANGER		PLAS P.C. PEN PL	PIPE COLUMN OR PORTLAND CEMENT PENETRATION PLATE
	CONCRETE. ESR-2811- GDB AND GDPS GAS-ACTUATED FASTENERS. ESR-2877 WOOD FRAMING CONNECTORS FOR MASONRY CONSTRUCTION.			PLYWOOD POUNDS PER SQUARE FOOT
	ESR-2811- GDB AND GDPS GAS-ACTUATED FASTENERS. ESR-2877- WOOD FRAMING CONNECTORS FOR MASONRY CONSTRUCTION. ESR-2920- CAST-IN-PLACE STRAP STYLE HOLDDOWNS (STHD'S) ESR-2992- PUNCHING SHEAR RESISTOR RAILS (PSRR)			POUNDS PER SQUARE INCH PRESSURE TREATED
	ESR-2920- CAST-IN-PLACE STRAP STYLE HOLDDOWNS (STHD'S) ESR-2992- PUNCHING SHEAR RESISTOR RAILS (PSRR) ESR-3006- QUIK DRIVE X SERIES SELF-DRILLING TAPPING SCREWS. ESR-3037- STRONG-BOLT 2 WEDGE ANCHORS. ESR-3046- STRONG-DRIVE SD SCREWS FOR STRUCTURAL CONNECTORS.			RADIUS REQUIRED
	ESR-3046 - STRONG-DOLL 2 WEDGE ANCHORS. ESR-3046 - STRONG-DRIVE SD SCREWS FOR STRUCTURAL CONNECTORS LISING SD-SERIES SCREWS	TORS.	RFTR REINF RET	RAFTER REINFORCE(ING)
NAII	ESR-3096- CONNECTORS USING SD-SERIES SCREWS. NAILING SCHEDULE, MINIMUM (TABLE 2304.9.1, 2010 C.B.C.):			RETAINING SPACED EQUALLY SPACED FOUNDLY FACH WAY
	•	3-8d	S.E.E.W. S.S. SHT	SPACED EQUALLY EACH WAY SELECT STRUCTURAL SHEET
2. 3.	JOIST TO SILL OR GIRDER, TOENAIL BRIDGING TO JOIST, TOENAIL EACH END 1"x6" SUBFLOOR OR LESS TO EACH JOIST, FACE NAIL WIDER THAT 1"x6" SUBFLOOR TO EACH JOIST, FACE NAIL 2" SUBFLOOR TO JOIST OR GIRDER, BLIND AND FACE NAIL SOLE PLATE TO JOIST OR BLOCKING, TYPICAL FACE NAIL SOLE PLATE TO JOIST OR BLOCKING, AT BRACED WALL PANELS TOP PLATE TO STUD, END NAIL	2-8d 2-8d	SIM SPECS	SIMILAR SPECIFICATIONS
4. 5.	WIDER THAT 1"x6" SUBFLOOR TO EACH JOIST, FACE NAIL 2" SUBFLOOR TO JOIST OR GIRDER, BLIND AND FACE NAIL SOLE BLATE TO JOIST OR BLOCKING TYPICAL FACE NAIL	3-8d 2-16d	SQ STAGRD	SQUARE STAGGERED
0. 7	SOLE PLATE TO JOIST OR BLOCKING, TTPICAL FACE NAIL SOLE PLATE TO JOIST OR BLOCKING, AT BRACED WALL PANELS TOP PLATE TO STUD FND NAII	3-16d PER 16" 2-16d	STD STL	STANDARD STEEL
8.	STUD TO SOLE PLATE	4-8d, TOENAIL OR	STR SYM	STRUCTURAL SYMMETRICAL
9. 10.	DOUBLED STUDS, FACE NAIL DOUBLED TOP PLATES, FACE NAIL	16d AT 24" O.C. 16d AT 16" O.C. 8-16d	T T.B.	TOP TOP OF BEAM
11.	BLOCKING BETWEEN JOISTS OR RAFTERS TO TOP PLATE, TOENAIL	3-8d	T.C. THK	TOP CHORD THICK
12. 13. 14	RIM JOIST TO TOP PLATE, TOENAIL TOP PLATES, LAPS AND INTERSECTIONS, FACE NAIL CONTINUOUS HEADER TWO PIECES	8d AT 6" O.C. 2-16d 16d AT 16" O.C. ALONG	T & B T & G	TOP AND BOTTOM TONGUE AND GROOVED
15	CEILING JOISTS TO PLATE. TOENAII	16d AT 16" O.C. ALONG EACH EDGE 3-8d	TS TYP U.N.O.	STRUCTURAL TUBE TYPICAL LINESS NOTED OTHERWISE
16. 17.	RIM JOIST TO TOP PLATE, TOENAIL TOP PLATES, LAPS AND INTERSECTIONS, FACE NAIL CONTINUOUS HEADER, TWO PIECES CEILING JOISTS TO PLATE, TOENAIL CONTINUOUS HEADER TO STUD, TOENAIL CEILING JOISTS, LAPS OVER PARTITIONS, FACE NAIL CEILING JOISTS TO PARALLEL RAFTERS, FACE NAIL RAFTER TO PLATE, TOENAIL 1" BRACE TO EACH STUD AND PLATE, FACE NAIL 1" BRACE TO EACH STUD AND PLATE, FACE NAIL	4-8d 3-16d	V, VERT W	UNLESS NOTED OTHERWISE VERTICAL WIDE FLANGE SHAPE
18. 19.	CEILING JOISTS TO PARALLEL RAFTERS, FACE NAIL RAFTER TO PLATE, TOENAIL	3-16d 3-8d	W W/ W/O	WITH WITHOUT
20.	1" BRACE TO EACH STUD AND PLATE, FACE NAIL	∠−8d 2−8d	W/O	WOOD

8. LAGBOLTS (AND SCREWS) SHALL BE PRE-DRILLED $\frac{1}{16}$ " LESS THAN SHANK DIAMETER TO FULL

ABBREVIATIONS:

SUPPLEMENTAL NAILING NOTES:

23. BUILT-UP CORNER STUDS

25. 2" PLANKS

24. BUILT-UP GIRDER AND BEAMS

1. ALL NAILS TO BE COMMON WIRE NAILS, WHERE BOX NAILS ARE

USED, THERE NUMBER MUST BE INCREASED BY 33%.

21. 1"x8" SHEATHING OR LESS TO EACH BEARING, FACE NAIL

22. WIDER THAN 1"x8" SHEATHING TO EACH BEARING, FACE NAIL

2. WHERE 2" MEMBER IS DETAILED USE THE NUMBER OF 16d SHOWN: FOR EXAMPLE:

AT EACH BEARING MEANS 3-16d

20d AT 32" O.C. AT TOP & BOTTOM AND STAGGERED 2-20d AT ENDS AND AT EACH SPLICE 2-16d

2 - 8d

3 - 8d

16d AT 24" O.C.

REVISIONS

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MONC INT DEP

comdev@mono.ca.gov www.monocounty.ca.gov 9-17-12 SCALE 3/4"=1'-0"

1 DRAWN TMS 12-008C

SHEETS